



# Pollution-based model predictive control of combined sewer networks, considering uncertainty propagation

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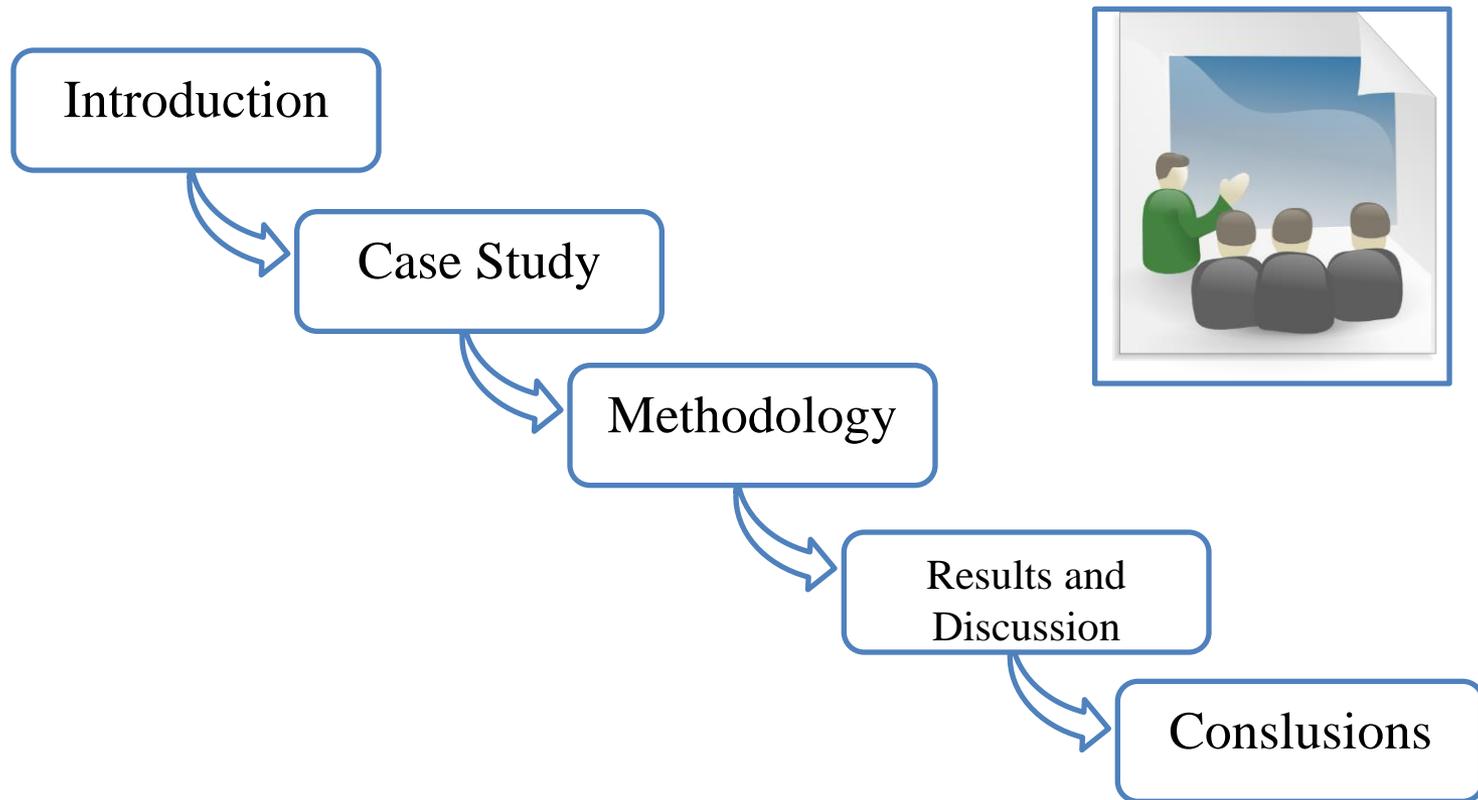
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# Outline:

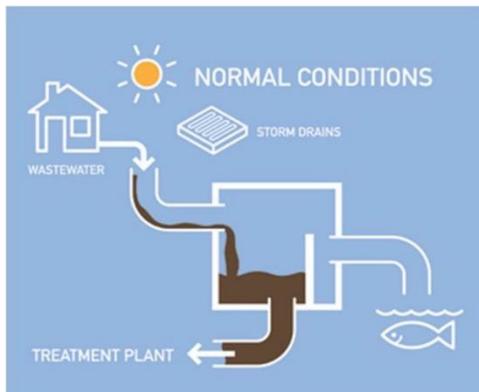


# Introduction

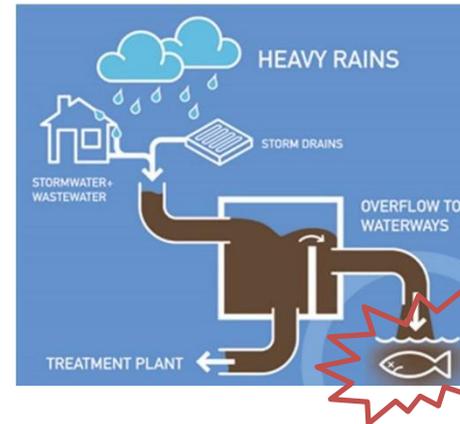


## Combined Sewer Network

Dry weather flow (DWF)



Wet weather flow (WWF)



Source: <http://whenitrains.commons.gc.cuny.edu/>

Real-time control (RTC)

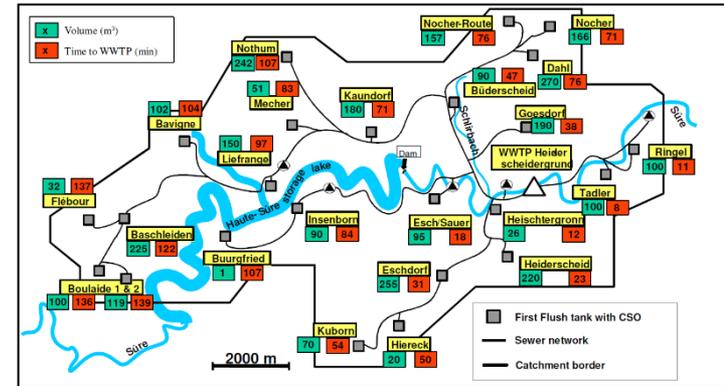
- Wastewater quantity and quality
- Fast (simple) model
- Uncertainty analysis
- No modification on the physical network

# Case Study

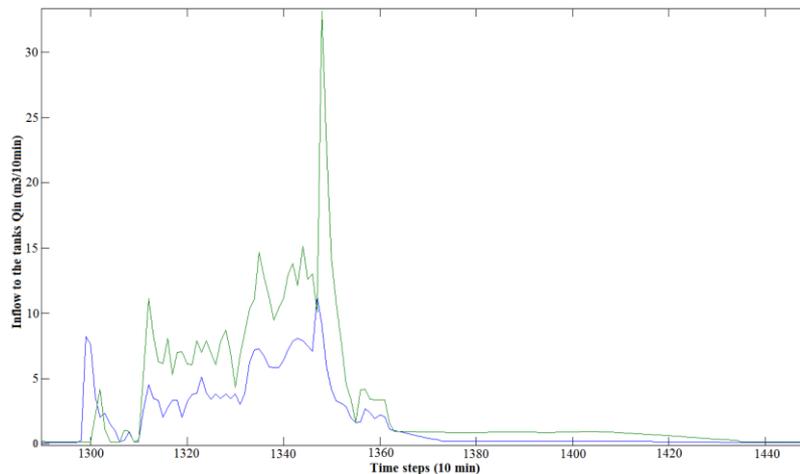


## ☐ Haute-Sûre catchment, Luxembourg

- Location: Northwest of Luxembourg.
- Capacity: 12000 population equivalents (PE). Future Plan: 24 Sub-catchments with 24 CSO tanks.
- In this research **only two** of CSO tanks are considered to test the controllers



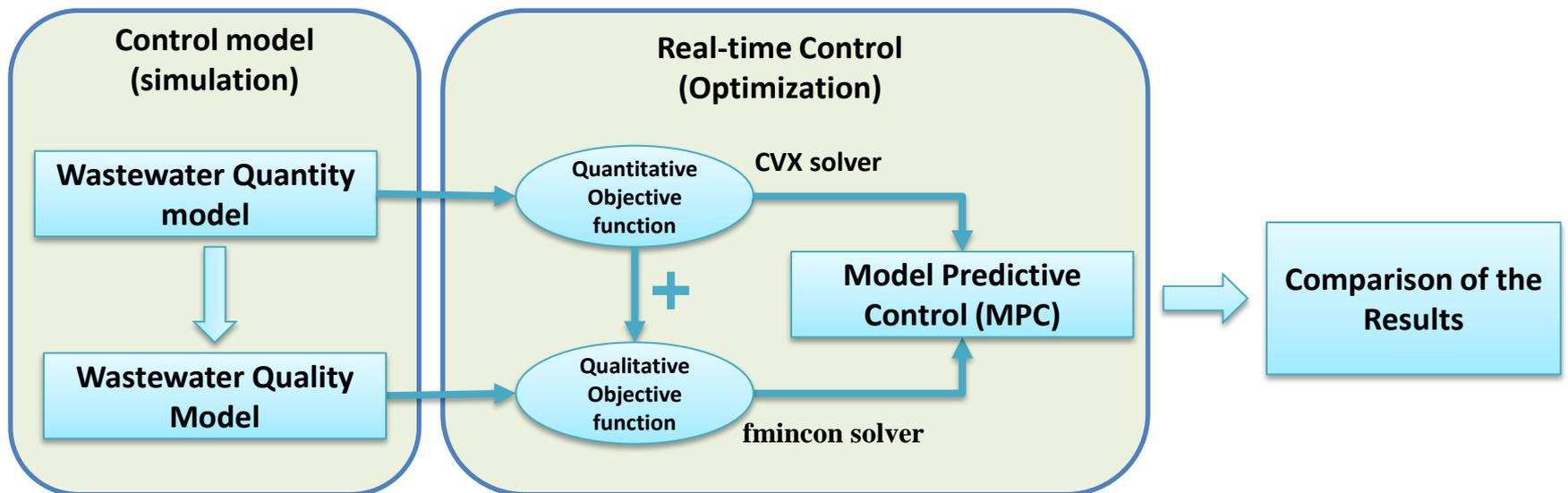
Haute-Sûre catchment (D. Fiorelli, G. Schutz, 2009)



Measured inflow to the tanks during the October 2002 rain scenario

Blue: Buderscheid CSO tank  
Green: Kaundorf CSO tank

# Methodology:

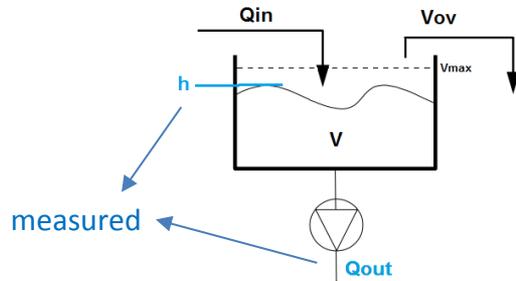


# Method: Wastewater Quantity model



## Simple tank model:

- ✓ Based on **conservation of volume** in the tank.



System variables

$$V(t) = V(t - \Delta t) + Q_{in}(t)\Delta t - Q_{out}(t)\Delta t - V_{ov}(t)$$

$$Q_{in}(t) = \left( \frac{V(t) - V(t - \Delta t) - V_{ov}(t)}{\Delta t} \right) + Q_{out}(t)$$

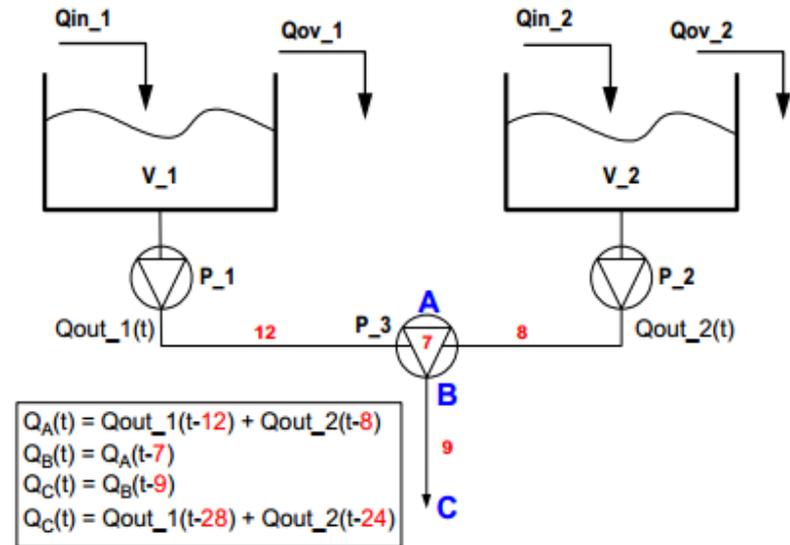
$$Q_{in}(t) = Q_{DW}(t) + Q_{WW}(t)$$

h: wastewater level in the tank (measured by sensor)  
 Q<sub>in</sub>: inflow  
 V<sub>ov</sub>: overflow volume

V: wastewater volume in the tank  
 Q<sub>out</sub>: outflow (measured and subject to control)  
 C: concentration of the pollutant load in the tank

## The flow in the network:

- ✓ Modelled using the **delay time** concept.



Time delay concept used in the network modelling.



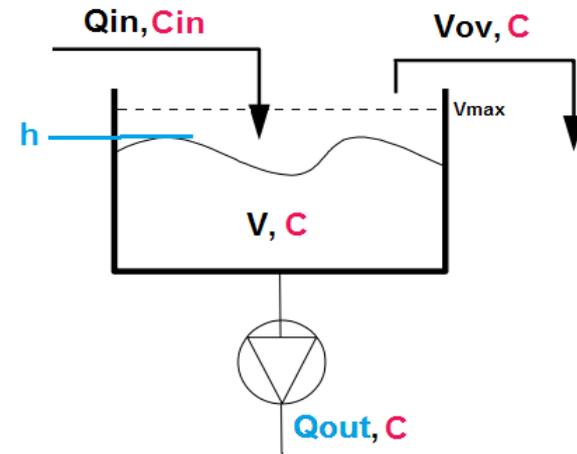
# Method: Wastewater Quality model



## Three main assumptions, there is:

1. Only **one global indicator** to reflect the pollution load;
2. Only a **simple dilution** effect in the tank;
3. **Homogeneous** concentration of the pollutant load in the tank 'C'

Taking into account previous equations and **mass balance** law:



System variables

$$m(t) = m(t - \Delta t) + m_{in}(t) - m_{out}(t) - m_{ov}(t)$$

$$C(t)V(t) = C(t - \Delta t)V(t - \Delta t) + C_{in}(t)Q_{in}(t)\Delta t - C(t - \Delta t)Q_{out}(t)\Delta t - C(t - \Delta t)V_{ov}(t)$$

$$C(t) = \frac{C(t - \Delta t)V(t - \Delta t) + C_{in}(t)Q_{in}(t)\Delta t - C(t - \Delta t)Q_{out}(t)\Delta t - C(t - \Delta t)V_{ov}(t)}{V(t - \Delta t) + [Q_{in}(t) - Q_{out}(t)]\Delta t - V_{ov}(t)}$$



# Method: Optimization



## Quantitative Objective Function

$$J = \sum_{n=t}^{t+Hp} \lambda \phi_1(n) + \beta \phi_2(n) + \alpha \phi_3(n)$$

$\Phi_1$ : To use the **storage capacity** of the network **homogenously**.

$$\phi_1(n) = \sum_{i=1}^N \left( V_i(n) - \frac{V_{imax}}{\sum_{j=1}^N V_{jmax}} \sum_{k=1}^N V_k(n) \right)^2$$

$\Phi_2$ : To keep the flow towards the **WWTP** as close as possible to the optimum operating reference value.

$$\phi_2(n) = \left( y_{ref}(n) - \sum_{i \in N_k^*} out_i(n - d_{i,k}) \right)^2$$

$\Phi_3$ : To minimize the **CSO** volume.

$$\phi_3(n) = \sum_{i=1}^N (ov_i(n) - NL)^2$$

$j = 1 \dots N_p$  : number of pipes in the network  
 $d_{i,k}$  : the transport time of the  $i^{\text{th}}$  tank to the destination tank  $j$   
( $j = k$  representing the arrival at the treatment plant).  
 $N_j^*$  : all the tanks draining directly to the destination  $j$ .  
 $NL$  : a negative number to have a linear objective function



# Method: Optimization



## Qualitative Objective Function

$$J = \sum_{n=t}^{t+H_p} \lambda\phi_1(n) + \beta\phi_2(n) + \alpha\phi_3(n) + \gamma\phi_4(n) + \mu\phi_5(n) + \sigma\phi_6(n) + \left(\frac{\delta}{\phi_7(n)}\right)$$

### Objectives:

$\Phi_4$  : to minimize the **overflowed mass**.

$\Phi_5$  : to minimize the **uncertainty** present in the concentration of the mass which is directly linked to the above mentioned goal  $\Phi_4$ .

$\Phi_6$  : to distribute the **pollutant mass** over the network **homogenously** which is in fact similar to  $\Phi_1$ .

$\Phi_7$  : to maximize the pollutant **mass** arriving at the **WWTP**

$$\phi_4(n) = \sum_{i=1}^N (C_i(n)Ov_i(n) - NL)^2$$

$$\phi_5(n) = \sum_{i=1}^N (U_i(n) - NL)^2$$

$$\phi_6(n) = \sum_{i=1}^N \left( C_i(n)V_i(n) - \frac{V_{i_{max}}}{\sum_{j=1}^N V_{j_{max}}} \sum_{k=1}^N C_k(n)V_k(n) \right)^2$$

$$\phi_7(n) = \left( \sum_{i \in N_k^*} C_i(n - d_{i,k})Out_i(n - d_{i,k}) \right)^2$$

### Constraints

The volume of wastewater in each tank, the outflow, and the wastewater contained in the pipes are all **positive variables** and limited by their **maximum capacity**



## Taylor series of first order approximation

- **Reasons:**
  1. because the qualitative model, although not linear, is **differentiable**.
  2. Besides, through measures in the real system for each variable in our simple model there is an idea about the **tolerance interval** in which it is located.

$$\text{Var}(C(t)) = U_{C(t)}^2 = \sum_{i=1}^6 \left( \frac{C(t)}{A_i} \right)^2 U_{A_i}^2$$

With:

$$A_1 = C(t-\Delta t), A_2 = V(t-\Delta t), A_3 = Q_{in}(t), A_4 = C_{in}(t-\Delta t), A_5 = Q_{out}(t), A_6 = V_{ov}(t-\Delta t).$$

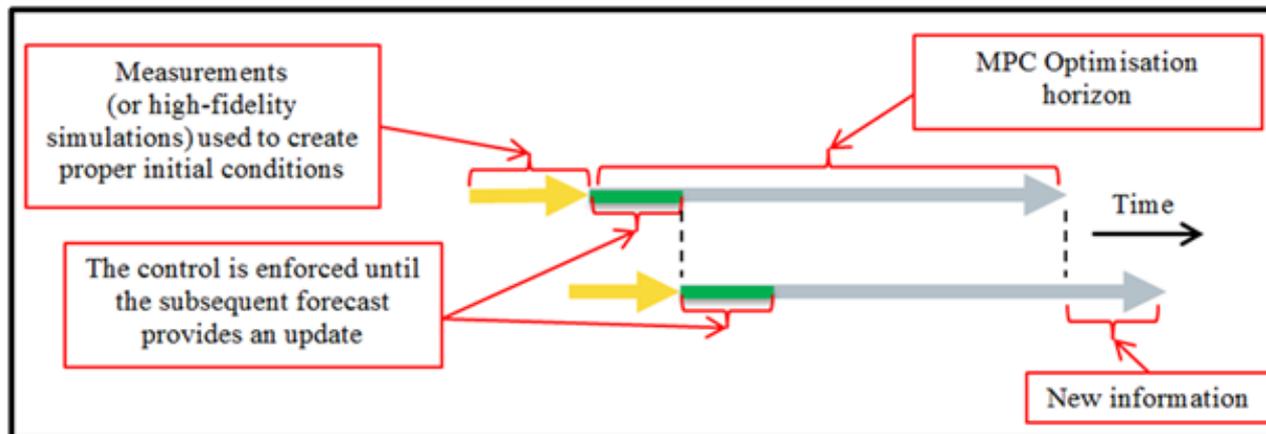
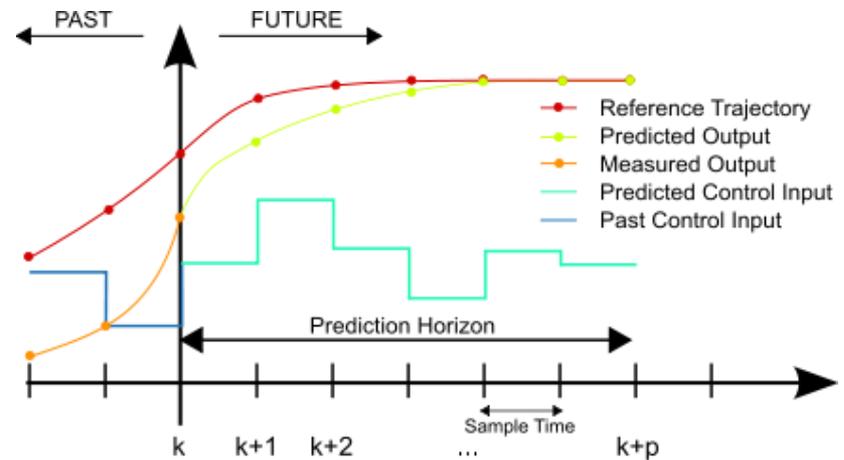
$$U_{C_{in}(t)}^2 = U_{C_{DW}(t)}^2 \left( \frac{Q_{DW}(t)}{Q_{in}(t)} \right)^2 + U_{C_{WW}(t)}^2 \left( \frac{Q_{WW}(t)}{Q_{in}(t)} \right)^2$$



# Model Predictive Control (MPC)



An **advanced real-time control** (RTC) approach which employs an **internal model** in order to forecast the behaviour of the given system in future over a finite time horizon (**receding horizon**). The principle of receding horizon is shown here:



# Results and Discussion

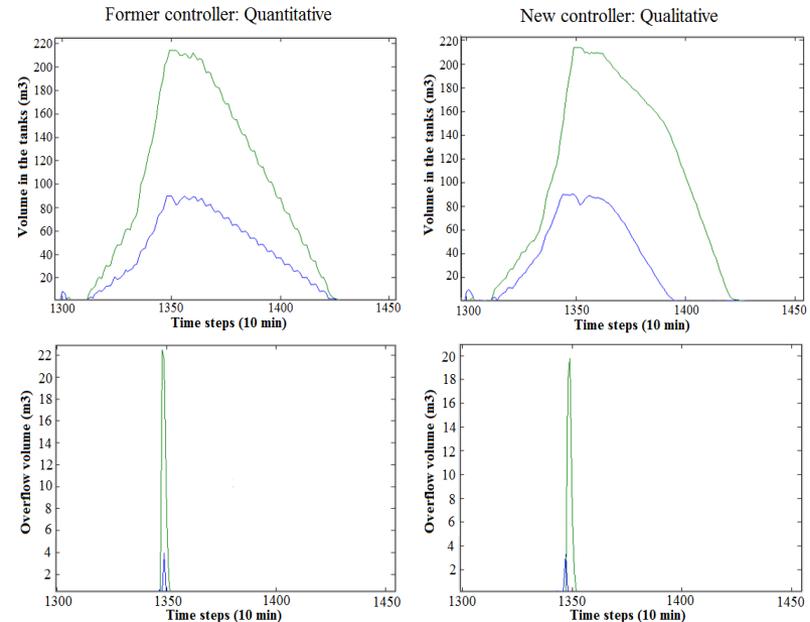


## A) Comparison of the controllers

quantitatively:

Volume in the tanks (m<sup>3</sup>) →

Overflow volume (m<sup>3</sup>) →



blue: Buderscheid CSO tank; green: Kaundorf CSO tank

Former controller: Quantitative	New controller: Qualitative
Overflow volume (Green): 52.8 m <sup>3</sup>	Overflow volume (Green): 45.9 m <sup>3</sup>
Overflow volume (Blue): 4.5 m <sup>3</sup>	Overflow volume (Blue): 3.5 m <sup>3</sup>
Total overflow volume: 57.3 m <sup>3</sup>	Total overflow volume: 49.4 m <sup>3</sup>

13.8% ↓

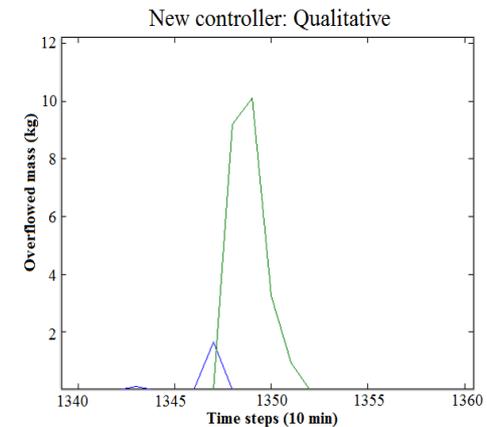
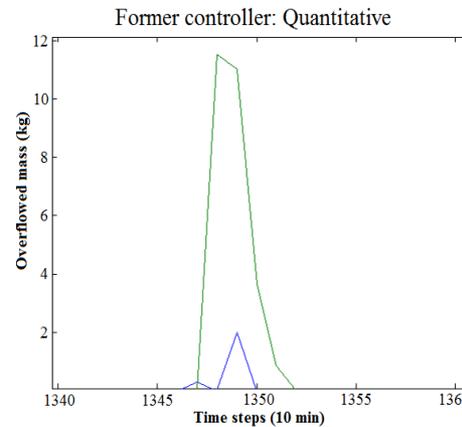
# Results and Discussion



## B) Comparison of the controllers

qualitatively:

Overflowed mass (kg)



blue: Buderscheid CSO tank; green: Kaundorf CSO tank

Former controller: Quantitative	New controller: Qualitative
Overflowed mass (Green): 27 kg Overflowed mass (Blue): 2.3 kg Total overflowed mass: 29.3 kg	Overflowed mass (Green): 23.5 kg Overflowed mass (Blue): 1.8 kg Total overflowed mass: 25.3 kg



The difference goes to the WWTP

# Conclusions



- The main idea was to understand if the quality-based controller can **improve** the **performance** of the quantity-based controller.
- the results showed a positive contribution of the quality-based controller in **decreasing the overflowed pollution mass as well as CSO volume** during the selected rain scenario.
- the new controller reduces the pollution load and overflow volume **without** the need to add **new physical elements** (e.g. sensors) to the system which are normally expensive to purchase and maintain.
- In fact, this is a very promising result and can be considered as a **'soft' solution** for combined sewer network management.



Thank you for your attention  
Any questions?

# Partners and Acknowledgements



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